

Chapter 4 Field Investigations

4-1. General

As previously shown in Table 2-1 for the feasibility phase and the preconstruction engineering and design phase of project development, geotechnical field investigations may be divided into two stages: Site Selection and Initial Field Investigations, and Foundation and Design Investigations. The need for this division depends on the planned size and complexity of the project. As a simple rule of thumb, if there is an envisioned need for test quarry and test fill operations, the two stages of field investigations are probably justified. Figures 4-1 and 4-2 show the required investigation steps for the two investigations stages. Guidance specific to the layout and conduct of the field investigations required to locate and design test quarry operations is provided in the following paragraphs.

4-2. Geologic Mapping

Surface geologic maps of potential test quarry sites should be prepared during the areal and site geotechnical mapping phase of the site selection investigations. The regional geologic maps, developed during the reconnaissance phase, commonly will have scales of 1:62,500 or larger. Depending on the size of the project, the areal (e.g. reservoir) geologic maps prepared during the current investigations phase may have scales of between 1:12,000 and 1:62,500. Structure site geologic maps would have scales of from 1:1,200 to 1:4,800. Potential test quarry sites should be mapped at scales comparable to larger scale site maps. The scales should be such that soil and rock type contacts can be shown, the location and shape of individual outcrop areas can be plotted, observed bedding and joint symbols can be plotted without cluttering the map, planned surface geophysical and core boring exploration plans can be shown, and planned excavation layouts can be shown. The outcrop mapping method is best for this type of work. An excellent reference for this type of mapping is Compton (1962). The geologic map produced and accompanying test data should include a complete lithologic classification description of the rock types present. In addition, the degree of weathering existing in the rocks at the site should be detailed.

4-3. Geophysical Investigations

Detailed guidance and information concerning the use of exploration geophysical methods and equipment are contained in EM 1110-1-1802. Of specific interest in quarry site explorations are: overburden depths, location and orientation of rock contacts, groundwater depths, seismic velocities, and rippability. Of the available surface geophysical exploration methods, seismic refraction, reflection profiling, and electrical resistivity are relatively economical and will provide the required information. Of the available borehole methods, up and downhole seismic, and electrical logging will provide the appropriate data. The spacing of surface seismic and electrical resistivity lines should be a function of the variability of top of rock elevations and rock-type distribution as inferred from the detailed geologic mapping.

4-4. Subsurface Explorations

The subsurface explorations stage can be carried out concurrently with or toward the end of the surface geophysical stage. Lagging behind the start of surface geophysics allows the results of the geophysical profiling to assist in the layout of the boreholes. Conversely, borehole information will assist in the interpretation of the geophysical data.

a. Core borings. For the purposes of exploring the sites of proposed rock excavations and test quarries, with rare exception, rock core borings are the most suitable drilling method. For most investigations in hard rock, "N" size diamond core borings which acquire a nominal 5.1 cm (2-in.) diameter core are satisfactory. For soft and/or highly fractured rocks, "H" size (nominal 7.6 cm (3-in.) diameter cores) or 10.2 cm to 14.0 cm (4 in. to 5.5 in.) diamond core borings may be necessary. As with the spacing of surface geophysics lines, the number and spacing of exploratory borings is a function of the anticipated rock variability. The borings should be arranged to facilitate the preparation of geologic cross sections with the borings at the ends of the anticipated cross sections outside the planned excavation limits; interpolation is much less risky than extrapolation. Borings should be located at the intersection of geophysical profiles to assist in correlation. Depending on the surface mapping results, it may be necessary to drill a number of angle holes to eliminate bias in borehole fracture surveys. Barring other

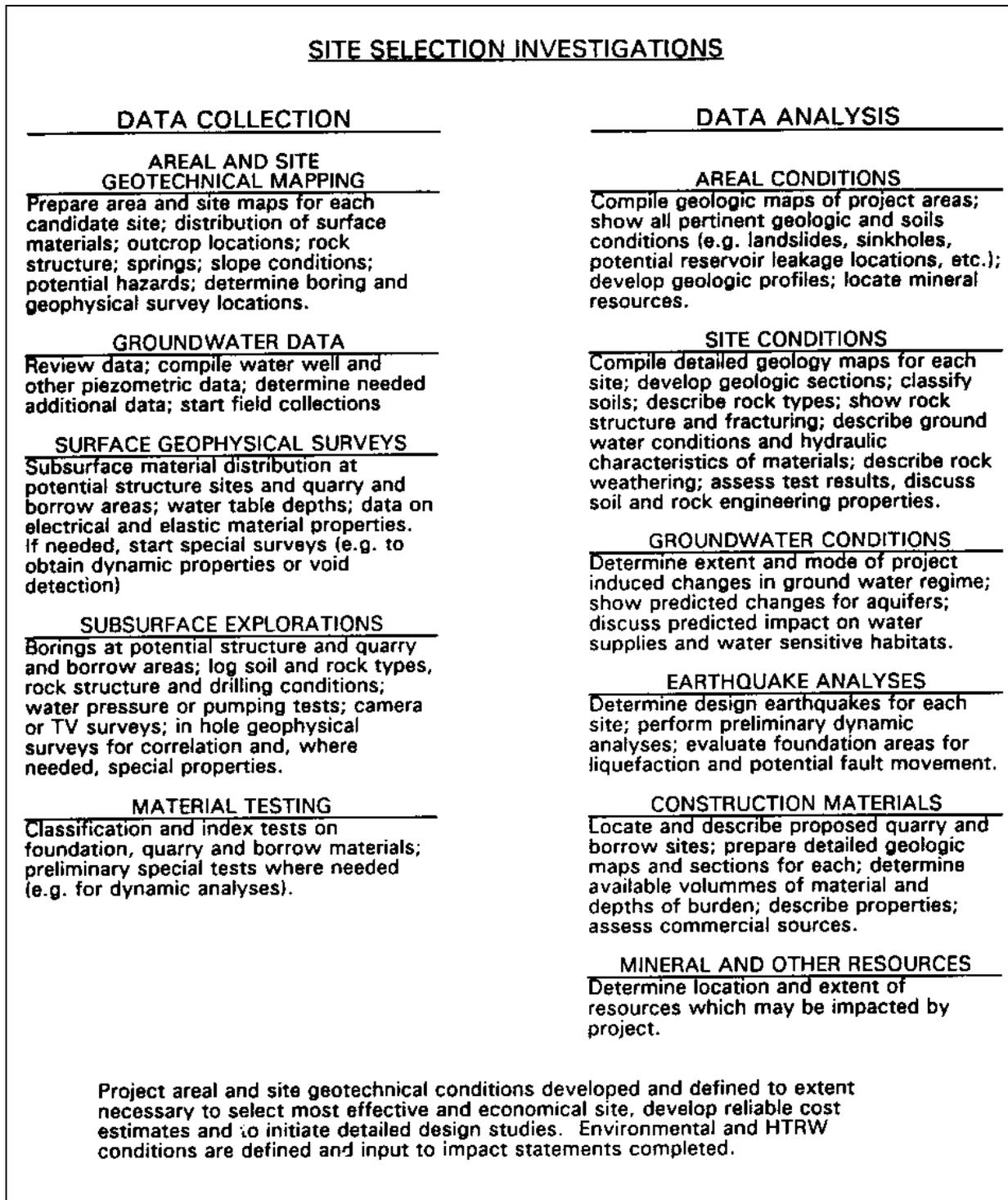


Figure 4-1. Schematic diagram of initial and site-selection investigations (adapted from EM 1110-1-1804)

considerations, or purposes for the borings, the depths of the core borings should be 1.25 to 1.33 times the depth

from the ground surface to the bottom of the planned excavation.

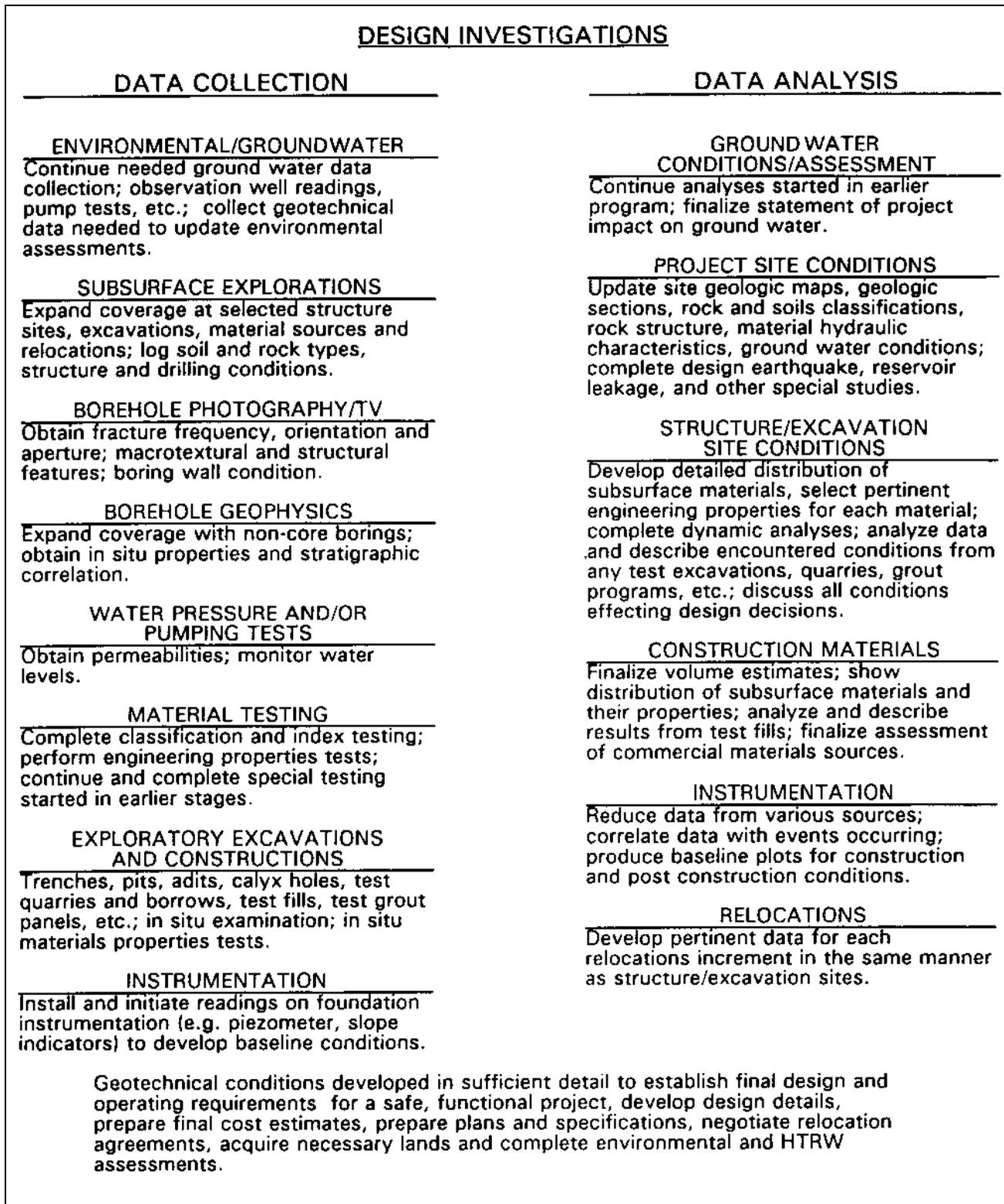


Figure 4-2. Schematic diagram for design investigations (adapted from EM 1110-1-1804)

b. Drilling, inspection, and sampling. General guidance for drilling, inspection, and core logging is contained in EM 1110-1-1804. The U.S. Bureau of Reclamation Engineering Geology Field Manual (1989) and Murphy (1985) contain comprehensive information on both core and soils logging and on rock mass descriptions. All of the rock descriptors recommended in the cited references are important to test quarry applications. Of particular importance are weathering, presence of clay or gouge seams, and the in situ gradation. Given a rock material of certain hardness and density, these types of descriptors will form the basis for estimates of waste and the need for and design of rock processing equipment. The degree of weathering should be described according to some standard such as that contained in EM 1110-1-1804. In situ fracture frequency and orientation can provide the information required to calculate in situ rock block-size distribution. In addition, correlations have been developed between rock-quality designation (RQD) and mean fracture frequency. The general use of RQD is treated in ETL 1110-1-145. When logging rock core, in addition to logging core loss and RQD, the geologist/inspector should note the depth and angle (with respect to borehole axis) of every identifiable fracture and note its genesis (joint, bedding plane, drill break, etc.). As a practical matter, when the fracture spacing is less than 3.0 cm (0.1 ft), that interval of core may be logged as "broken." This data will allow the prediction of in situ rock block-size distributions. In addition, it will be of value for the geologist/inspector to note those parameters which are used in currently popular rock mass classification systems such as "RMR" or "Q" systems (ASTM 1988). The use of rock mass classification systems is discussed further in Chapters 6 and 7. Information and guidance on sampling and sample preservation of soils and rock core are contained in EM 1110-1-1906, and ASTM Designations: D 4220 (ASTM 1994a) and D 5079 (ASTM 1994b). Sample

preservation for moisture content is generally not necessary in hard, crystalline rocks. Such rocks generally exhibit intact rock material porosities less than one percent and moisture content is inconsequential to densities and other parameters. In softer sedimentary or chemical precipitate rocks, the porosities are sufficiently great that moisture content does affect bulk densities and other parameters. In these rocks, the geologist/inspector should select and preserve representative samples for moisture content determinations.

c. Borehole examination and in-hole testing. Borehole examination and in-hole testing includes borehole photography, TV and sonic imaging, borehole geophysics, hydraulic or water pressure testing, water table measurements, and in-hole deformation or jacking tests. Guidance and information on these methods are contained in EM 1110-1-1802, EM 1110-1-1804, the Corps of Engineers Rock Testing Handbook (USAEWES 1993), and the U.S. Bureau of Reclamation Engineering Geology Field Manual (1989). The borehole geophysical methods most pertinent to test quarry explorations have been described in paragraph 4-3. Tests to determine in situ stresses would be warranted only if there were indications of abnormally high horizontal stresses which might affect excavation slope stability and markedly affect rock breakage. For test quarry explorations, water pressure testing normally would not be necessary. Water table measurements are necessary for excavation slope and dewatering design purposes. For test quarry explorations, borehole photography, TV, and sonic imaging are the most important measurements. Because they provide detailed information on rock structure, these measurements will provide input to design analyses of probable rock waste, in situ rock block-size distribution, trial blast patterns and loading, and rock excavation slope stability calculations and slope design.